

1 **Journal of Advanced Transportation**

2 **Impact on Network Performance of Probe Vehicle Data Usage: an** 3 **Experimental Design for Simulation Assessment**

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13 **Abstract**

14 Probe-based technologies are proliferating as a means for inferring traffic states. Technological
15 companies are interested in traffic data for computing the best routes in a traffic-aware manner
16 and they also provide real-time traffic information with certain temporal accuracy. This paper
17 analyses and evaluates how data provided by a fleet of probe cars can be used to develop a
18 navigation service and how the penetration rate of this service affects a set of city-scale KPIs
19 (Key Performance Indicators) and driver KPIs. The case study adopts a model-driven approach
20 in which microscopic simulation emulates real-size fleets of probe vehicles that provide positions
21 and speed data. What is noteworthy about the modelling behaviour is that drivers are segmented
22 according to their knowledge of network conditions for selected trips: experts, regular drivers
23 and tourists. The paper presents and discusses the modelling approach and the results obtained
24 from an experimental Barcelona CBD model designed to evaluate the penetration rates of probe
25 vehicles and route guidance. An analysis of the simulation experiments reveals remarkable links
26 among city-scale KPIs, which – from a multivariate point of view – is a novelty. A simulation-
27 based framework for results analysis and visualization is also introduced in order to simplify the
28 simulation results analysis and easily visualize OD paths for driver segments.

29 **Introduction**

30 Mobility is a key component in urban areas and should be addressed as part of a complex system,
31 since it is a non-isolated component that strongly interacts with all the other components.

32 There is wide consensus about the brand new family of services that will be enabled by advances
33 in inter-vehicular communications. From the early considerations of equipped vehicles as a
34 network of mobile sensors (1) to more recent surveys (2), the capabilities for effectively
35 monitoring traffic conditions have been studied. In urban areas, vehicles equipped with onboard

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1 sensors are expected to reach high concentrations in the near future. See (3) for a survey of
2 works and applications related to traffic state monitoring.

3 In a parallel line, researchers have been motivated by advancements in vehicle positioning
4 (onboard sensors) and in wireless communications that support Vehicle-to-Vehicle (V2V) and
5 Vehicle-to-Infrastructure (V2I) applications. They have investigated how the collected data could
6 be used to generate travel time information (see (4, 5)), and this research could be considered
7 complementary to the estimation of links or path travel times from GPS probe vehicles.
8 Summarily speaking, two complementary approaches are explored in the literature: (6) presents
9 an overview of the statistical approaches; and this was later improved upon by (7), who
10 combined GPS data with data from other travel time sources. A variant of these statistical models
11 that exploit GPS data is analysed in (8) to identify in the network paths whose travel times are
12 estimated in (9). An academic approach relying on traffic flow theory has been adopted by (10,
13 3) for the estimation of fundamental variables in arterials or urban motorways using Probe
14 Vehicle Data (PVD) and Edie's definitions (11). V2I applications under incident conditions have
15 been developed and route guidance strategies evaluated by (12).

16 In the context of exploiting mobile data, a popular trend (13, 14) explores the use of handover
17 information in cellular networks to estimate the traffic level of service, although they note
18 important limitations on the overall performance.

19 The use of PVD has been investigated in some research projects such as Mobile Millennium or
20 CarTel (1, 15, 16), which included a pilot traffic-monitoring system using the GPS in cellular
21 phones to gather traffic information, process it, and distribute it back to phones in real time.
22 Products and companies performing mobile crowdsourcing (Google Traffic, INRIX and
23 TomTom Traffic) allow for real-time data gathering. Machine learning applications for
24 estimating travel time delays due to road work from GPS data have been proposed by (17).

25 Route guidance impact on travel time, safety and environment have been intensively investigated
26 (12, 18–21), usually in relation to their benefits under incident conditions and while
27 simultaneously conducting quantitative assessments of the potential impacts of real-time routing
28 guidance and advisory warning messages to guided vehicles. Some other authors have analysed
29 different types of reactive (22) and proactive route guidance (23) policies using simulation, but
30 the elaboration of traffic state estimation is simplistic.

31 The aim of this paper is to present a simulation-based platform that allows modelling several
32 penetration rates for a fleet of PVD vehicles that feed travel time estimation between Points of
33 Interest (POIs) and several penetration rates of route-guidance for connected vehicles while
34 considering driver behaviour and route choice models. Travel time estimates between POIs from
35 PVD are critical inputs in Kalman filtering formulations, which some authors have addressed in
36 the context of dynamic OD matrix estimation (24): travel time availability and reliability from
37 PVD guarantees a simplified linear formulation approach (25). The resulting analysis platform
38 can be easily adapted to any microscopic traffic simulator with the required extended
39 functionalities, and it has been tested using a model of the Barcelona Business District developed
40 and calibrated in the past for previous projects (3). The conceptual framework is described first,
41 and the section following that describes the simulation experiments, in which a large fleet of
42 PVD vehicles are accounted for according to penetration rates (V2I) and additional factors

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1 considered in the experimental design. The next section provides an analysis of the results based
2 on a set of network and driver KPIs, which are jointly considered by applying multivariate
3 analysis techniques. The paper ends with our conclusions.

4 **Simulation Testbed Framework**

5 Figure 1 shows the simulation testbed framework, which is comprised of an Execution
6 Controller, a Traffic Simulation Module, a Results Processing Module and a Visual Analytics
7 Module. The visualization and analytics tool has been implemented using the Shiny (26) web
8 application framework for R, which simplifies the development of simulation results analyses by
9 incorporating it into interactive web applications. The methodological framework can be
10 implemented with any traffic simulation software by using the utilities that enable it to integrate
11 – via API – the user-defined applications that implement the system’s required functions, thus
12 guaranteeing the transferability of the approach.

13 The Traffic Simulation Module was programmed using API extensions, and the its components
14 are: emulation of PVD; auto demand split into vehicle classes according to driver type definition;
15 estimation of lane and link travel times from data collected from PVD emulation; and
16 customization of route choice models for guiding drivers according to the implemented
17 navigation strategies. The assessment of navigation strategies (route-guidance) is not the goal of
18 the current work, although it is the aim of a follow-up project.

19 The simulation results analysis is performed by two fundamental and independent components:
20 Results Processing and Visual Analytics modules. The Results Processing in Figure 1 covers all
21 pre-processing of data automatically. In this way, the Visual Analytics Module can use the results
22 of the traffic simulation environment directly without the need for a manual update.

23 **Traffic Simulation Module**

24 The traffic simulation component includes a microscopic traffic simulator and a set of custom
25 modules and functions that were developed using API extensions. In addition to the
26 functionalities already described at the beginning of this section, it also generates time-dependent
27 system tables as well as link and lane traffic data by driver class.

28 In this work, an Aimsun (27) model was available from previous projects. Aimsun functional
29 architecture and the interaction libraries (Aimsun API) support the extended modelling utilities
30 that are required.

31 The exchange of information between the API applications and the microsimulator can be made
32 at every simulation step (0.5 sec). The programming languages in which Aimsun provides its API
33 are C++ and Python. While Python is used to easily collect some of the data, C++ is needed for
34 emulating the Probe Vehicles due to performance reasons.

35 **Probe Vehicle Data Emulation**

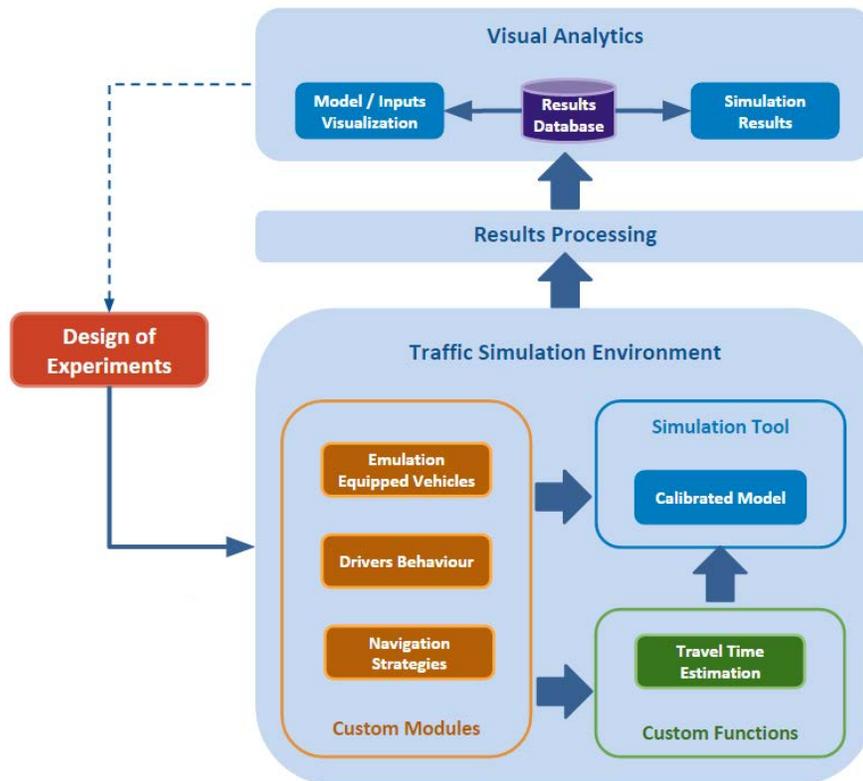
36 This work assumes that the V2I and V2V technology is on board in probe cars. In a previous
37 study by some of the authors, field test data from a fleet of 3 probe cars is discussed for

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1 Barcelona's CBD (3). Collected and filtered data were used to calibrate the emulation of PVD by
2 an API included in the Traffic Simulation Module (see (3)). Only basic vehicle sensors and no
3 frontal camera data were used previously in the probe vehicles. The API-Extension for emulating
4 PVD depends on on-board sensors and technological specifications.

5 The aim of the current work is to emulate the 'real-time data' of probe car data that is used in
6 connected car guidance systems within different levels of probe car penetration. To this end, a
7 reduced set of sensors for probe cars has been assumed, thus allowing data for vehicle position
8 and speed at each simulation step.

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Figure 1: Simulation Testbed Architecture.

12 Execution Controller

13 A simulation experiment consists of N replication executions that are launched from a controller,
14 and each of them is set up by pre-processing and is then post-processed at the end of the
15 simulation in order to collect the results generated from each replication. KPIs are grouped into
16 either Network KPIs or Driver-Type KPIs.

17 Results Processing and Visual Analytics Modules

18 The results of the simulation executions are stored on a server. The Results Processing Module
19 (see Figure 1) contains the set of processes that are in charge of finding new execution results

[Type here]

1 from recently performed simulation replicas and applying to them the post-processing that the
2 Visual Analytics Module needs.

3 The Visual Analytics Module in Figure 1 covers the visualization of input data, the model details
4 and the simulation results from any replication execution in the experimental design. It also
5 consists of two implementations: that of the visualization application and the one that serves
6 access to the web. The visualization is implemented with R-Shiny framework (26). For
7 performance reasons, C++ is used in some of the application processes.

8 **Modelling Issues**

9 Drivers are split into six groups according to guidance availability and their knowledge of
10 network and traffic conditions. This work emphasizes driver behaviour modelling issues, which
11 have not been considered in related papers in the literature (12, 28, 20–23, 29).

12 **Driver Behaviour**

13 The first group is Expert drivers, i.e., those who know the network and historic traffic conditions
14 for the selected horizon of study. They are modelled with route choice selection and proportions
15 by following experienced travel times that both satisfy dynamic user equilibrium (DUE) (30) and
16 assume a historic demand pattern. DUE paths and proportions are loaded into the simulation
17 environment from a pre-calculated binary file. The second group is Regular drivers, i.e., those
18 with knowledge of the network and historic traffic conditions for recurrent trips (50% randomly
19 selected), but who use the main streets based on free-flow for non-recurrent trips (50%). Expert
20 and Regular drivers exhibit driving characteristics related to the car-following model, such as
21 reaction times, desired speed and acceptance of speed limitations. According to the calibrated
22 profile of Barcelona drivers: Reaction Time (1.0s); Reaction Time at Stop (1.35s); Reaction
23 Time at Traffic Light (1.35s); Speed Acceptance and Minimum Inter-Vehicular Distance are
24 assumed to be truncated normally distributed, with the former having mean-1.1, sd-0.1, min-0.9
25 and max-1.3, and the latter having mean-1.0m, sd-0.3m, min-0.5m and max-1.5m. The third
26 group is Tourist drivers, who have limited knowledge of network and traffic conditions and use
27 K-shortest paths based on free flow conditions and main streets. Tourist drivers behave roughly
28 with a 25% increment in reaction times, means and limits (same standard deviation), and they
29 strictly adhere to the speed limits, while Minimum Inter-vehicular Distance is truncated normally
30 distributed with mean-1.25m and sd-0.1m – between 0.75 and 1.5 m. Finally, Guided drivers
31 constitute a design-dependent proportion for any Expert, Regular or Tourist driver class, and they
32 are modelled with a 100% acceptance of navigation advice.

33 **Travel Time Estimation**

34 The selected approach is based on the first model presented in (5) for estimating lane travel
35 times. The time-window concept – which can be viewed as a rolling horizon for updating
36 lane/link travel time estimates – is critical for understanding the approach. The time-window
37 interval is a design factor in the conducted simulation experiments because the penetration rate
38 of PVD leads to lane (link)-level vehicle data availability. Hence, as the time-window interval is
39 increased, the percentage of lanes (links) with available data and data units also increases.

[Type here]

1 Position and speed data provided by the PVD can be emulated for each simulation step (0.5 sec)
2 or at any multiple of the interval. Having PVD provided every 2 sec has been finally assumed for
3 the experimental study described in this paper in order to limit the computational burden of
4 executing replications. Thus, detection interval is a configurable parameter set to 2 sec for the
5 current study. Many alternative possibilities could be considered to develop travel time
6 estimators, but that is not the aim of this work and it is considered a topic for further research.
7 The particular proposal implemented here considers three cases for every lane:

8 • **Case 1:** “No PVD in the last window”. In this situation, the lane travel time value is
9 the same as the lane travel time of the most recent time window. If no data is available for this
10 lane, then the lane travel time is set to free-flow travel time.

11 • **Case 2:** “PVD from just one car in the last window”. To compute the travel time in
12 this case, consider the following:

13 – TT_1 : Estimated travel time for the fraction of the lane until the first detection of the
14 vehicle. TT_1 is computed by dividing the length of this fraction by the probe
15 vehicle’s instantaneous speed at the first observation in this lane for the considered
16 detection interval, which gives a time measure.

17 – TT_L : This value is the difference in seconds between the first and last detection
18 intervals.

19 – TT_2 : This is computed in the same way as TT_1 . We divide the distance from the last
20 observation at the beginning of the next section in the trajectory by the probe
21 vehicle’s instantaneous speed in the last observation (detection interval) of the time-
22 window.

23 Finally, the travel time of the considered lane is $TT_1 + TT_L + TT_2$.

24 • **Case 3:** “PVD available from more than one car in the last time window”. In this
25 case, the weighted travel time of the lane is $\frac{1}{n} \sum_{v=1}^n w_v TT_v$, where TT_v is the travel time of a car
26 computed in the same way as in Case 2, and w_v is the fraction of the section length in $[0,1]$ that v
27 probe car is detected in the lane.

28 Hence, the weights w_v handle the lane changing behaviour of vehicles. In other words, if a
29 vehicle travels in the slower lane at first, then switches to the faster lane and travels for some
30 time before switching back to the slower lane and then exiting the segment, then 3 independent
31 PVD detections are assumed, which thus prevents underestimation of the travel time for the
32 slower lane and overestimation of the travel time for the faster lane.

33 The estimation of link travel times from PVD is obtained as the mean of travel times for streams.
34 Lane travel time estimates are combined into stream travel times according to the turning
35 movements that are allowed.

36 **Navigation Strategies**

37 A navigation application is modelled as being available to a common percentage in driver
38 classes, since PVD can be used to estimate travel times in network links and thus, travel times

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1 between OD POIs. Route guidance assessment is not the aim of this paper (see (18) for an
2 interesting discussion about the topic); thus, three routing strategies are implemented according
3 to the possibilities of the AIMSUN (27) platform:

- 4 • Free-flow K-shortest paths when no probe fleet is available.
- 5 • Stochastic Route Choice: lane-based, according to instantaneous traffic conditions
6 (lane travel times) as inferred from data provided by PVD.
- 7 • Stochastic Route Choice: link-based, according to instantaneous traffic conditions
8 (link travel times) as inferred by PVD.

9 Stochastic Route Choice (SRC) modelling accounts for the estimated instantaneous K-shortest
10 travel time paths, as defined by either user-defined link costs (link travel time estimates from
11 probe car data) or user-defined stream costs (lane travel time estimates are combined and
12 together they constitute stream travel times). Also, 100% re-routing across the trip is enabled
13 (every Time Window interval 1.5, 3 or 6 min) in order to account for dynamic and instantaneous
14 travel-time-based route choice. K is set to 3 in all the experiments. A proportional SRC is
15 calibrated to determine the probability of selecting a route for a K-Shortest travel time path that
16 is calculated using instantaneous travel times. Thus, choice probability P_k of a given alternative
17 path k is,

$$18 \quad P_k = \frac{CP_k}{\sum_{l \in K} CP_l} \quad (1)$$

19 where CP_i is the cost of path i . The proportional parameter is set to $\alpha=1.2$ after calibration in
20 order to consider the probability as inversely proportional to path costs.

21 The key point is that a dynamic (real-time) routing strategy for connected cars (guided cars) is
22 applied from PVD-derived travel times. Travel time estimates used in K-shortest path
23 calculations might depend either on lane-based travel time estimates or overall link-based travel
24 time estimates, both of which rely on PVD sent to a centralized sub-system.

25 Travel times between POIs can be inferred from OD path travel times and route-choice
26 proportions. They can then feed Kalman filtering formulations to estimate dynamic OD matrices,
27 as proposed by the authors.

28 **Collected KPIs**

29 Default statistics in traffic microsimulation platforms are usually very rich. Statistics have been
30 collected every 90s and stored in an SQLITE database for each replication. Driver KPIs collected
31 for each Expert, Regular and Tourist driver type – either Guided or Non-Guided – are:

- 32 • Mean Travel Time to cover 1 km (s/km)
- 33 • Mean Speed per vehicle (km/h)
- 34 • Mean Delay while covering 1km (s/km)
- 35 • Mean Travel Distance per vehicle (km)
- 36 • Mean Travel Time per vehicle (min)

[Type here]

1 Network KPIs are global statistics (all driver classes, buses included) and, over the whole
2 simulation horizon, they are:

- 3 • Total Travel Distance (km)–**ttdis**
- 4 • Total Travel Time (h)–**ttt.h**
- 5 • Fuel Consumption (l)–**fuelc**
- 6 • Total CO2 emissions (kg)–**co2**
- 7 • Total NOx emissions (kg)–**nox**
- 8 • Mean Travel Time to cover 1 km (s/km)–**mtt.s.km**
- 9 • Mean Delay Time while covering 1km (s/km)–**mdelay.s.km**
- 10 • Density (veh/km)–**density**
- 11 • Mean Speed (km/h)–**mspeed**
- 12 • Mean Flow (veh/h) (throughput measure)–**mflow**
- 13 • Throughput Rate (%) completed trips divided into total demand–**thruptrate**

14 **Design of Experiments**

15 The selected scenario is the Barcelona CBD, known as “L’Eixample” (see (3) for details), which
16 comprises 7.46 km² and 250,000 inhabitants. The horizon study is 1h, accounting for 42,500
17 trips. Passenger car demand is modelled as 15-min time-sliced demand whose Origin-Destination
18 pattern reproduces the 9-10h morning period in L’Eixample. The model includes the description
19 of the 50 bus-routes operating in the area and accounts for frequencies and stops for
20 boarding/alighting.

21 **Design Factors**

22 Factors considered in the **design of the simulation experiments** are:

- 23 • **Driver Type Distribution** (factor TD) into Expert-Regular-Tourist for:
 - 24 ○ Base level: 40-50-10.
 - 25 ○ Alternative levels: 20-70-10, 40-40-20 and 60-20-20.
- 26 • **Guidance Penetration** (factor GP). Percentage of cars that are connected cars whose
27 route choice decisions follow those advised by a Navigation Tool fed by PVD:
 - 28 ○ Base level 0%.
 - 29 ○ Alternative levels: 10%, 20%, 30%, 70%, 80%, 90% and 100%.
- 30 • **Demand Pattern** (factor DP) into 4 levels – 0%, 10%, 20% and 30% – referring to a
31 perturbation of the historic demand pattern in OD pairs belonging to the fourth percentile trip
32 distance (according to Manhattan distance). They account for 42,500, 44,600, 46,860 and 48,600
33 trips, respectively.
 - 34 ○ Base level: 0% means historic demand pattern.
 - 35 ○ Alternative levels: 10%, 20% and 30% increments.
- 36 • **Probe Vehicle Penetration percentage (PVD factor)**, modelled as common to any
37 driver type into 4 levels: 0%, 10%, 20% and 30%. Base level is 0%. It indicates route guidance
38 based on free-flow travel times. An additional Ground Truth level consisting of travel time
39 estimates based directly on ‘simulated Ground Truth’ was also included in some initial
40 experimentation.

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- **Time-Window length** (factor TW) is the rolling horizon interval considered for the estimation of traffic variables from PVD:
 - Base length is 3 min.
 - Alternative lengths are 1.5 min and 6 min. TW is not affected when 0% PVD is set.
- **Navigation Strategy** (factor NS) models driving recommendations based on either lane-level or link-level PVD when PVD is available. Base level is lane-level, but it is not affected when 0% PVD is set.

Table 1: Relative (%) and Absolute errors (min) for long trips (Manhattan distance origin to destination centroid over 3km) when 5 replicas are considered in the base scenario at 90%, 95% and 99% confidence levels. ¹

Average Travel Time		Number of replicas 5										
min		TG	TNG	TOURIST	RG	RNG	REGULAR	EG	ENG	EXPERT	GUIDED	NON - G
Mean		9.54	9.88	9.80	9.45	9.74	9.67	9.53	9.98	9.88	9.49	9.85
Abs error	90	0.28	0.22	0.22	0.11	0.17	0.15	0.11	0.17	0.14	0.11	0.17
	95	0.36	0.29	0.29	0.14	0.23	0.20	0.14	0.22	0.19	0.15	0.22
	99	0.60	0.48	0.47	0.23	0.38	0.33	0.23	0.37	0.31	0.25	0.37
Rel error %	90	2.90%	2.23%	2.24%	1.12%	1.79%	1.57%	1.12%	1.71%	1.46%	1.20%	1.73%
	95	3.77%	2.91%	2.92%	1.45%	2.34%	2.04%	1.46%	2.23%	1.91%	1.56%	2.26%
	99	6.26%	4.82%	4.84%	2.41%	3.88%	3.38%	2.42%	3.69%	3.16%	2.59%	3.74%

The critical KPI, from a driver satisfaction point of view, is considered to be mean travel time (min), but also for researchers interested in the assessment of travel times between POIs inferred from PVD. A detailed analysis on the base scenario for all design factors found that when N=5 replications, this facilitates a global 5% relative precision in mean travel time (min) at 95% confidence for any driver type, while the greatest absolute error was about 1/3 min (Tourist). Table 1 shows absolute/relative errors at different confidence levels for driver types when 5 replicas are considered. Thus, the base scenario is set at factor levels: factor TD 40-50-10, factor GP 0%, factor DP 0%, PVD factor 0%. Therefore, TW and NS levels are irrelevant.

Running the full factorial design is unfeasible for computational reasons, since $3,072 \times 5 = 15,360$ replications would be needed. Therefore, the first set of experiments (first round) was constrained in order to identify non-aliased factor main effects according to the Fedorov algorithm (31) for optimal designs: 29 experiments were given (thus, 145 replications were

¹ Column name legend: TG-Tourist Guided, TNG-Tourist Non-Guided, RG-Regular Guided, RNG-Regular Non-Guided, EG-Expert guided and E-NG-Expert Non-Guided. Tourist, Regular and Expert Columns indicate Driver classes (averaging Guided/Non-Guided cars) and Guided/Non-G columns indicate mean over Driver classes.

[Type here]

1 executed, each one taking around 2h on an Intel Core i7-4790 CPU (frequency of 3.6GHz)-4
2 cores-8GB DDR3 Memory and Windows 8.1 (x64 system)).

3 A second round of simulation experiments for the base level 40-50-10 in TD factor and 3 min for
4 Time-Window factor (TW) were launched to quantify the most significant factors found in the
5 first round of simulations. In decreasing order of importance: Demand Pattern (DP), PVD
6 penetration, Guidance Penetration (GP) and Navigation strategy factors (NS). The second round
7 design consisted of 140 new replications.

8 **Results and Discussion**

9 Network KPIs are affected by design factors, either when gross effects or net effects are
10 considered (i.e., elaborated from the linear model for each global network KPI in all design
11 factors). A gross effect factor indicates the factor impact when it is considered alone, and this is
12 the mean of KPIs at different levels of the selected factor. A net effect for a factor means the
13 effect of all other factors – except for the selected factor – are taken into account before
14 calculating the *net* effect for each level of the selected factor. A heatmap representation has been
15 chosen to summarize the gross and net effects of design factors on network KPIs (see Figure 2).
16 Clearly, demand pattern (DP) and guidance penetration (GP) strongly affect all network KPIs.
17 The gross effects of driver-class configuration (TD) impact Total Travel Time and emission
18 KPIs. The net effects of PVD, NS, TW and TD factors decrease when the other factors are
19 considered, while the net effects of the DP and GP factors remain the same or are even magnified
20 for some KPIs, such as those related to emissions and total travel distance. There is a reduction in
21 the net effects of DP on the mean travel time for covering one km (mtt.s.km) and the mean delay
22 by km (mdelay.s.km) once the remaining factors are controlled for. Dendrograms of KPIs and
23 design factors are grouped from bottom to top according to the most similar KPIs. They start by
24 assigning each item to its own cluster, and proceed to find the closest (most similar) pair of
25 clusters and then merge them into a new single cluster. DP and GP effects are similar on the
26 factor side, although on the KPI side similarities arise between emission KPIs, travel time
27 (mtt.s.km) and delay (mdelay.s.km) for covering one km.

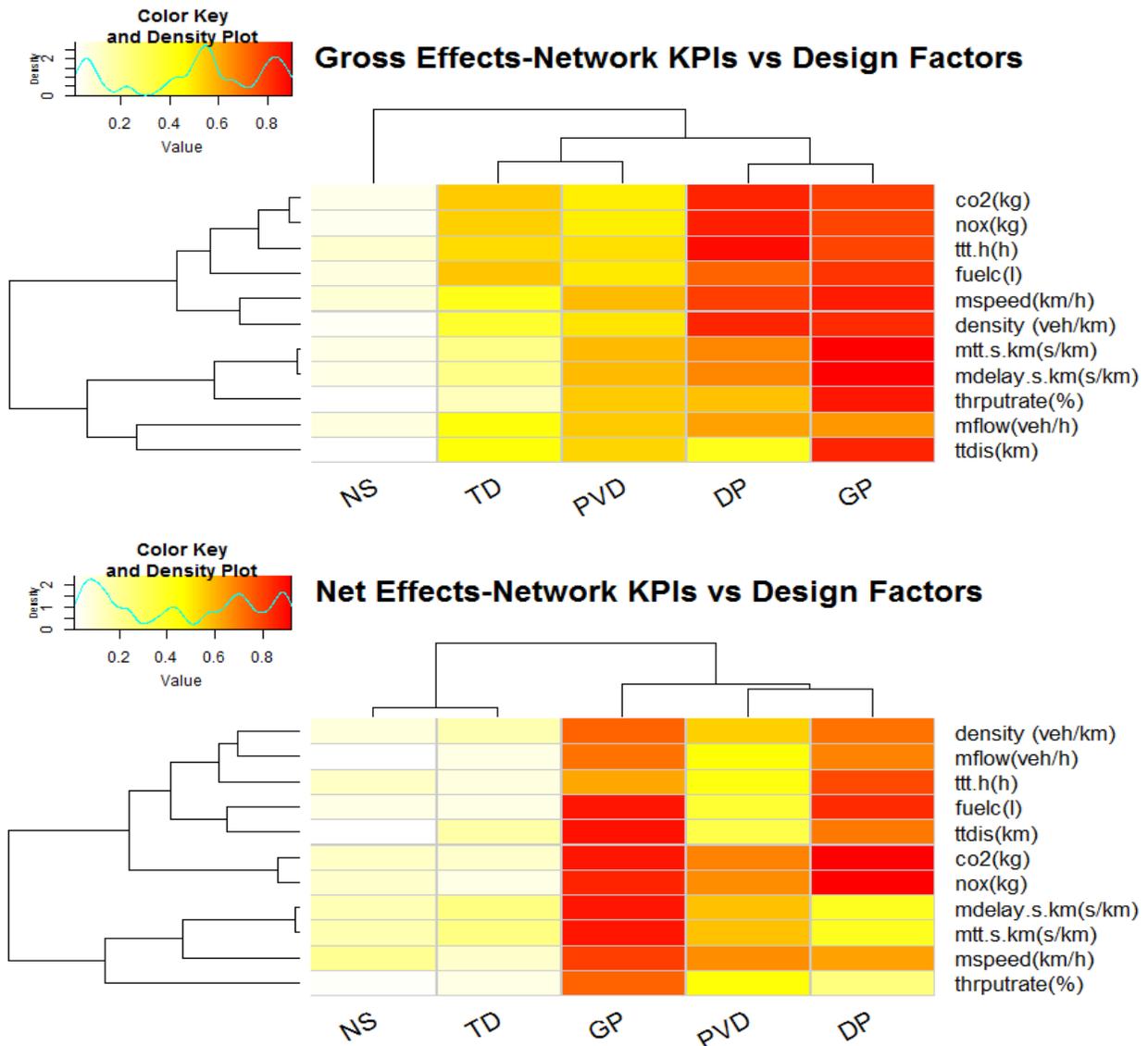
28 Network KPIs are numeric variables with different scales and internal correlation; thus a
29 normalized PCA (Principal Component Analysis) is applied, which explains almost 95% of the
30 total inertia (variability in KPIs) in the first factorial plane. Each axis in the plane models a
31 ‘hidden variable’ that combines the original ones. The first and second axes explain, respectively,
32 56% and 39% of the total variability. On the other hand, the projection of the cloud of replicas
33 and KPIs onto the first factorial plane reveals the meaning of hidden variables (see Figure
34 3(top)).

35 The first factorial axis (see Figure 3 (top)) is a size axis that is positively associated with the
36 most contributive variables, such as the total travel time (ttt.h), density, and the CO2 and NOx
37 emissions, as well as DP factor (total demand). In the opposite direction of mean speed, this axis
38 clearly represents quantity of trips (total demand), and it is dominated obviously by the DP
39 factor. Once the total number of trips is controlled for, the network performance is represented on
40 a second axis that is orthogonal to the first one. The network throughput flow (mflow) and the
41 total travelled distance (ttdis) are positively correlated, indicating an increase in total distance
42 when throughput flow increases. This total travelled distance is also more positively related to

[Type here]

1 fuel consumption than to CO2 and NOx emissions. On the negative side of the second axis, there
 2 is also a positive correlation with density when we look at the mean delay (mdelay.s.km) and the
 3 mean travel time (mtt.s.km) needed to cover a km.

4 According to the overlapped representation of ellipses around the centres of demand pattern
 5 levels: low increments over historic demand are located to the left of the interpreted size axis 1;
 6 high increments go to the right; and the largest variability range changes from axis 1 to 2, thus
 7 increasing congestion. Clearly, the second axis is a hidden variable that indicates quality of
 8 service in terms of congestion (better in the positive part of axis 2). Once the demand pattern
 9 factor DP has been clearly identified, the rest of the factor levels are located in the diagram to
 10 gain interpretability on the quantity-quality hidden variables plane.



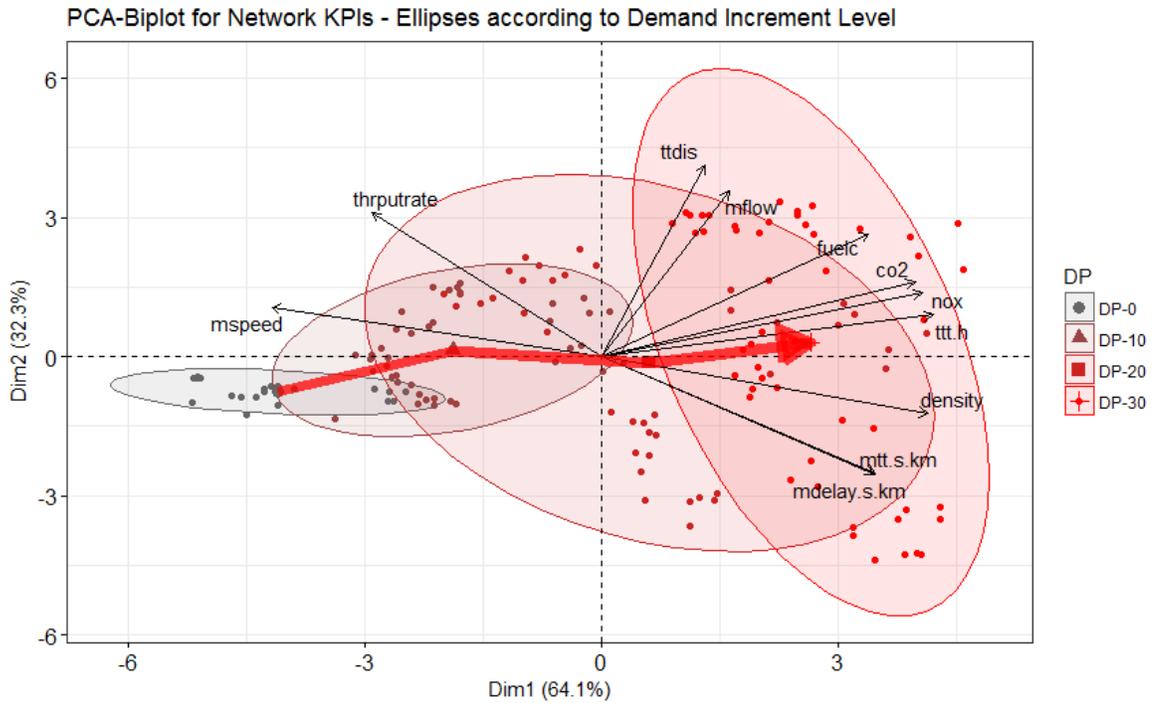
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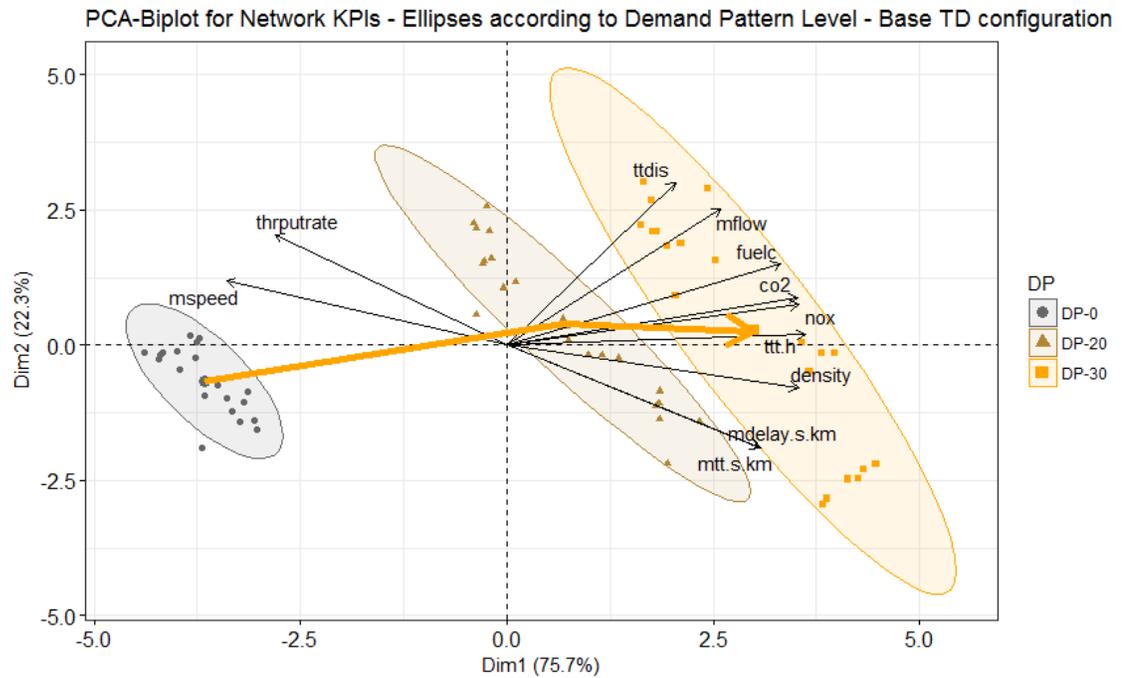
13 Figure 2: Gross (top) and Net (bottom) effects of design factors on Network KPIs. All replicas. Colour proxy
 14 developed from (partial) Spearman correlation. First round of simulations.

15

[Type here]



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3 Figure 3: Biplot of Principal Component Analysis of Network KPIs. 90% confidence ellipses around Demand levels.
 4 (top) All configurations-First round of simulations and (bottom) Only Base Driver Class. Second round of
 5 simulations.

6 In Figure 3 (bottom), the meaning of the axis quantity-quality is reinforced for the second round
 7 of simulations, which has more homogeneous KPI results in terms of variability. Clearly, the
 8 second axis is a latent congestion-level axis where delay and mean time for covering one km
 9 (mdelay.s.km and mtt.s.km) are 180 degrees opposite of the throughput rate. Once the demand

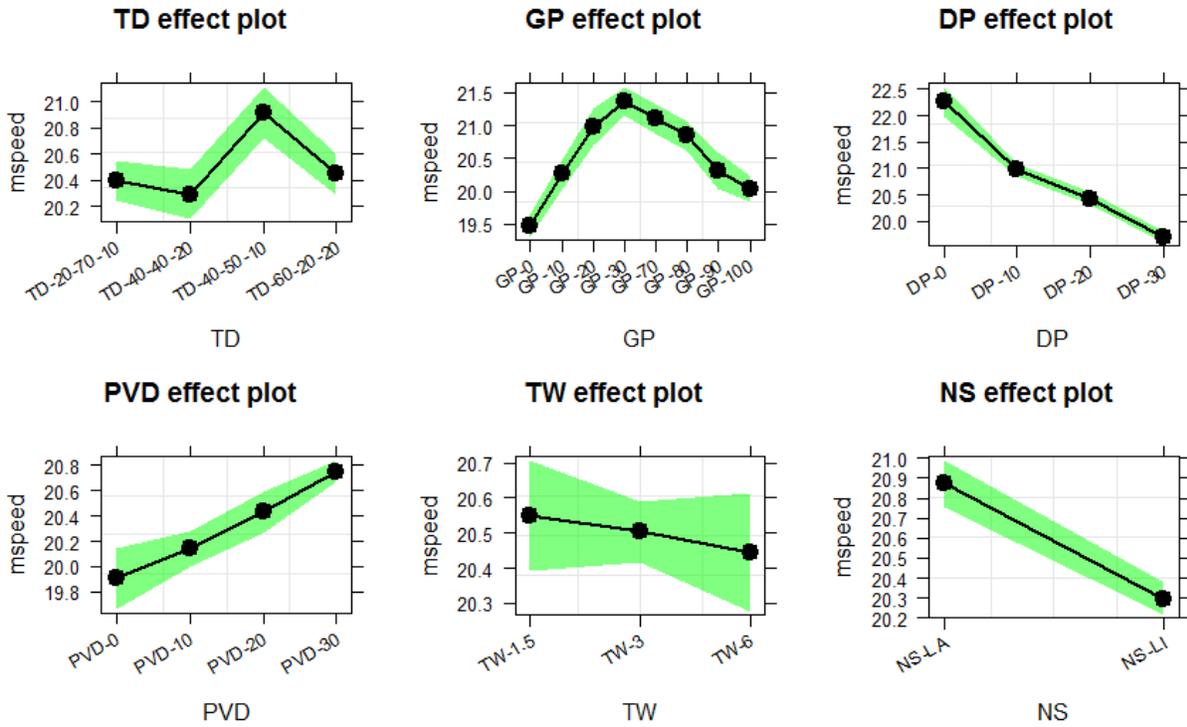
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1 factor DP has been clearly identified, the rest of the factor levels are located in the diagram to
2 gain interpretability from the quantity-quality latent plane (Figure 3 (top)).

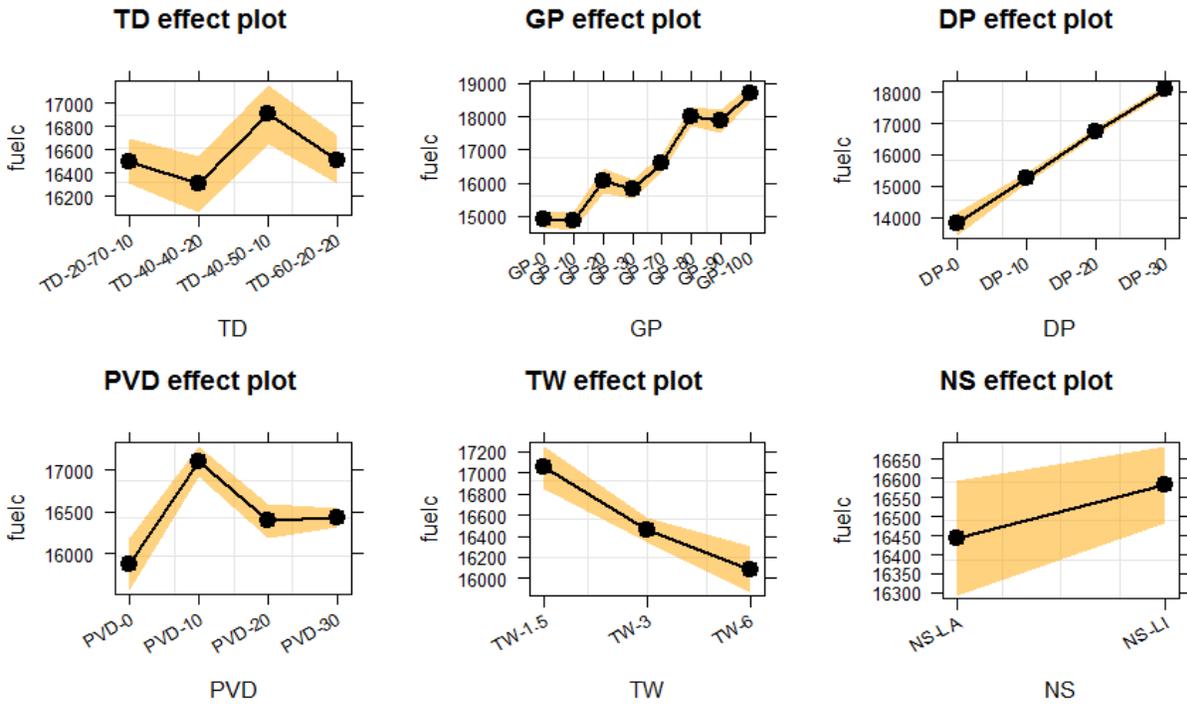
3 For the second round of simulations in the base level of driver-type configuration (TD), the
4 meaning of the quantity-quality latent plane is reinforced (Figure 3 (bottom)). Blue and yellow
5 arrows in Figure 3 Left to Right join the gravity centres for DP level ellipses from low to high
6 demands.

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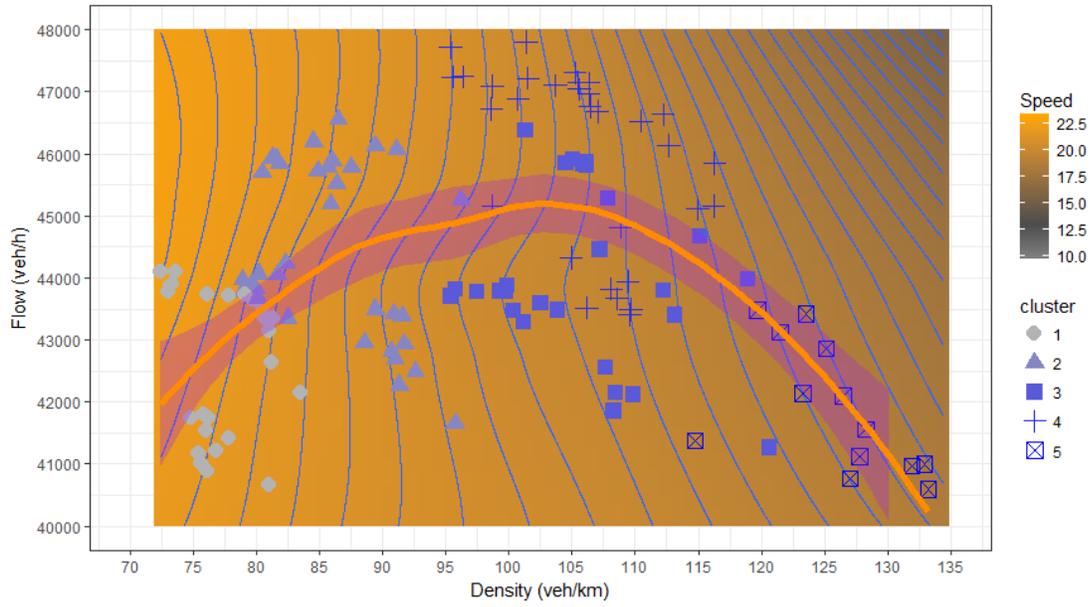
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Figure 4: Speed (top) and Fuel Consumption (bottom). Network KPI net effects on design factor levels. All Driver Class Configurations. First round of simulations.

5

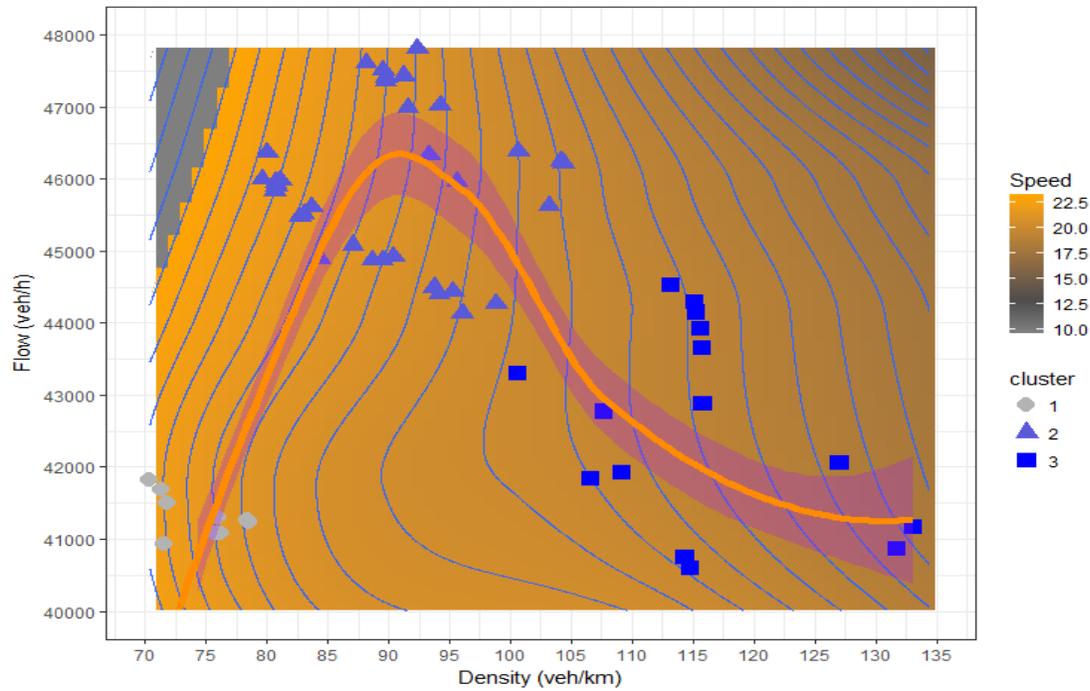
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Flow - Density - Speed



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Flow - Density - Speed



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Figure 5: Flow-Density-Speed diagram with replications grouped by unsupervised clustering techniques. (top) First round of simulations. (bottom) Second round of simulations.

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Table 2: Mean Speed (top) and Mean Travel Time (bottom) for Driver Type 40-50-10 Expert-Regular-Tourist (second round of simulations) according to DP and PVD factors.

Mean Speed (km/h)		PVD Factor - Probe Vehicle Data Penetration						
DP	Driver Type	Base-0%	10%		20%		30%	
0%	Expert-G		22.27	1.28%	22.50	2.34%	22.51	2.39%
	Expert-NG	21.99	22.24	1.16%	22.55	2.54%	22.70	3.23%
	Regular-G		22.06	1.65%	22.33	2.88%	22.37	3.09%
	Regular-NG	21.70	21.94	1.07%	22.26	2.55%	22.43	3.35%
	Tourist-G		21.09	2.31%	21.41	3.82%	21.56	4.58%
	Tourist-NG	20.62	20.86	1.17%	21.20	2.84%	21.41	3.84%
20%	Expert-G		20.91	3.14%	21.45	5.79%	21.46	5.84%
	Expert-NG	20.28	20.48	0.98%	21.22	4.66%	21.37	5.40%
	Regular-G		20.81	4.36%	21.29	6.74%	21.39	7.27%
	Regular-NG	19.94	20.15	1.05%	20.90	4.82%	21.08	5.68%
	Tourist-G		19.96	4.94%	20.50	7.79%	20.70	8.83%
	Tourist-NG	19.02	19.26	1.26%	19.99	5.07%	20.12	5.79%
30%	Expert-G		19.95	5.39%	20.48	8.21%	20.62	8.93%
	Expert-NG	18.93	19.38	2.39%	20.15	6.43%	20.34	7.47%
	Regular-G		19.95	6.89%	20.33	8.96%	20.52	9.98%
	Regular-NG	18.66	19.15	2.61%	19.83	6.26%	20.08	7.60%
	Tourist-G		19.34	8.35%	19.71	10.43%	19.92	11.59%
	Tourist-NG	17.85	18.30	2.53%	18.98	6.33%	19.22	7.65%

3

Mean Travel Time (min)		PVD Factor - Probe Vehicle Data Penetration						
DP	Driver Type	Base-0%	10%		20%		30%	
0%	Expert-G		5.50	-0.47%	5.37	-2.80%	5.34	-3.22%
	Expert-NG	5.52	5.37	2.82%	5.14	-6.96%	5.24	-5.13%
	Regular-G		5.47	0.53%	5.35	-2.72%	5.33	-3.08%
	Regular-NG	5.50	5.35	2.71%	5.12	-6.98%	5.21	-5.20%
	Tourist-G		5.62	0.85%	5.47	-3.48%	5.48	-3.28%
	Tourist-NG	5.66	5.51	2.77%	5.26	-7.17%	5.36	-5.43%
20%	Expert-G		6.34	1.25%	6.02	-6.25%	6.01	-6.36%
	Expert-NG	6.42	6.35	1.09%	5.83	-9.23%	5.89	-8.28%
	Regular-G		6.31	2.18%	6.04	-6.48%	6.03	-6.60%
	Regular-NG	6.46	6.33	1.88%	5.83	-9.75%	5.91	-8.41%
	Tourist-G		6.43	2.12%	6.14	-6.52%	6.16	-6.33%
	Tourist-NG	6.57	6.45	1.92%	5.95	-9.50%	6.03	-8.25%
30%	Expert-G		6.70	2.65%	6.40	-6.99%	6.42	-6.68%
	Expert-NG	6.88	6.79	1.24%	6.32	-8.18%	6.33	-8.02%
	Regular-G		6.73	3.07%	6.45	-7.17%	6.45	-7.04%

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Regular-NG	6.94	6.81	- 1.89%	6.31	-9.04%	6.39	-8.03%
Tourist-G		6.75	- 4.48%	6.53	-7.51%	6.56	-7.07%
Tourist-NG	7.06	6.97	- 1.37%	6.45	-8.69%	6.53	-7.54%

1

2 Marginal net effects of mean speed and fuel consumption have been analysed for design factor
3 levels in Figure 4. It is noteworthy that a guidance penetration (GP factor) increment of up to
4 30% benefits mean speed, but the mean speed tends to decrease for guidance levels over 30%,
5 since guidance strategies are merely reactive. Nevertheless, as demand increases (DP factor),
6 mean speed decreases almost linearly. However, the fuel consumption KPI tends to increase as
7 either GP or DP increases.

8 Clearly, increasing the guidance penetration (GP) has a positive effect on performance in terms
9 of mean speed (up to a 30% limit), but also on fuel consumption KPIs, as shown in Figure 4; and
10 lane-level guidance overperforms the results of link-level guidance for the two selected KPIs in
11 Figure 4.

12 A diagram in the style of a macro fundamental diagram (32) is presented in Figure 5 (top). It
13 considers flow, density axis and contour curves for speed, and it concerns the first round of
14 simulations, with replications grouped according to unsupervised clustering techniques based on
15 factorial space distances. Five clusters are identified by hierarchical classification rules. Note that
16 there are two clusters (3 and 4) at the break point of congestion: one over the curve indicating a
17 large throughput (network performance, cluster 4); and the second (cluster 3) is mostly under the
18 curve, which indicates lower performance (throughput). The difference between these two
19 groups relies on the 30% DP with 70%, 80% and 90% GPs in cluster 4 over the curve, while
20 cluster 3 points exhibit 20% DP and low GP levels. Cluster 5 belongs to 30% DP, with no
21 situations in which serious damage to network performance occurs due to guidance being
22 provided. In Figure 5 (bottom), simulations in cluster 2 have 20% DP, so congestion is found for
23 GP over 30%. Routes that are experienced according to recurrent conditions (historic demand)
24 are not valid when an increasing demand appears; therefore, the reactive navigation strategies
25 tend to maintain the level of service by increasing total travel distance and fuel consumption.
26 Otherwise, the level of service decays and NO_x and CO₂ emissions increase.

27 Finally, Table 2 (top) presents interesting figures for the means of speed and trip travel time
28 (according to Driver KPIs) in different scenarios for the second round of simulations. No
29 guidance is the reference for each class of driver and DP level. Under base demand (0% DP), all
30 drivers tend to increase their speed moderately when probe vehicle penetration grows (guidance
31 is available for a variable part of the population according to experimental design), but non-
32 guided expert and regular driver speeds are better than those for guided drivers (since recurrent
33 congestion condition are present). As more demand is injected into the network (DP 20% and
34 30%), guided vs. non-guided expert differences increase in favour of guided ones, but even non-
35 guided vehicles from any class benefit from incremental PVD penetration. In Table 2 (bottom),
36 the mean trip travel time for guided vehicles is always less than trip travel time for non-guided
37 vehicles when demand increases (20% and 30% DP), when low PVD penetration is present (10%
38 PVD) and the overall benefit within each driver class is enhanced as PVD increases.

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1 Nevertheless, as PVD increases, non-guided drivers benefit from shorter mean travel times. A
2 detailed analysis of the results suggests that reactive navigation strategies should be considered
3 in detail, because longer distance routes are produced for guided vehicles and, therefore, speed is
4 higher for guided vehicles while their final trip travel times are not reduced since they follow
5 longer routes.

6 **Conclusions**

7 The research carried out was based on an approach consisting of a general framework and
8 simulation architecture for emulating and evaluating penetration rates of probe car fleets and
9 navigation services for a subset of drivers. The contribution of this paper relies on a detailed
10 simulation of driver classes. Reactive navigation strategies were calculated according to travel
11 times estimated from PVD, and they have been shown to be advantageous for guided and non-
12 guided cars up to a certain level of around 30% of guided vehicles for any PVD penetration.
13 Nevertheless, reactive guidance increases the level of service at the expense of increasing guided
14 trip lengths (the overall trip travel time decreases), thus increasing fuel consumption. This is
15 unhelpful in terms of sustainability. Additionally, when demand increases and the level of
16 service for drivers decreases, it is worth using route guidance for a fraction of the vehicles
17 because this reduces NOx and CO2 emissions (despite the longer distances and consumption).
18 We can definitively say that any assessment of mobility services must consider several KPIs,
19 since a simple increment in driver speed or a reduction in travel time does not automatically
20 revert positively in terms of sustainability (fuel consumption and emissions reduction). In
21 recurrent traffic conditions, navigation devices are not suitable for expert drivers, but those
22 drivers do benefit from their being used by the other cars. Furthermore, because travel times
23 involved in guidance have been estimated from a fleet of PVD and even their lowest tested
24 penetration rate is 10%, the overall effect on network KPIs is positive. Hence, after extensive
25 analysis of the results obtained for several KPIs, it can be concluded that OD travel times
26 between POIs inferred from PVD appear to be reasonable inputs for simplifying dynamic OD
27 matrix estimation formulations that are being developed by the authors in an ongoing work (also
28 proposed in (24)). Finally, PVD penetration should represent a non-negligible percentage of
29 drivers, since 20-30% of the results are consistently better for any KPI at the network level than
30 those obtained for a 10% PVD Penetration rate (a fleet of 4,250 probe vehicles in the base
31 scenario).

32 **Conflicts of Interest**

33 The authors declare that there is no conflict of interest regarding the publication of this paper.

34 **Funding Statement**

35 This research was funded by TRA2016-76914-C3-1-P Spanish R+D Programs and by Secretaria
36 d'Universitats-i- Recerca -Generalitat de Catalunya- 2017-SGR-1749.

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1 Acknowledgements

2 Throughout this work, the authors have benefited from the support of inLab FIB team at UPC
3 and the suggestions of Jaume Barceló (Emeritus Professor at UPC).

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